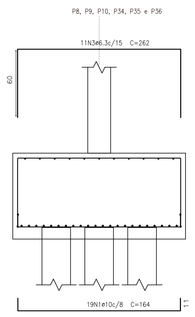
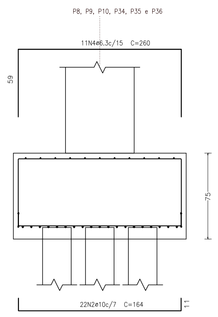
P8, P9, P10, P34, P35 e P36

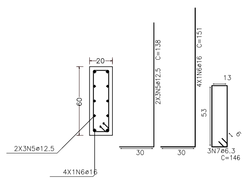
P8, P9, P10, P34, P35 e P36



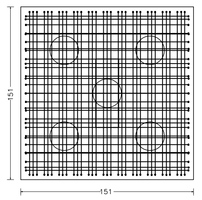
P8, P9, P10, P34, P35 e P36



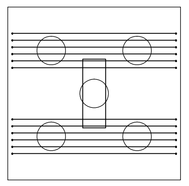
P8, P9, P10, P34, P35 e P36

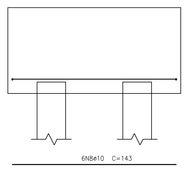


Estacas: D25

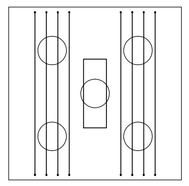


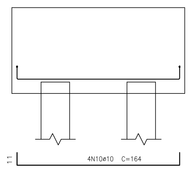
Viga paralela X



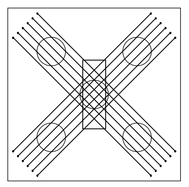


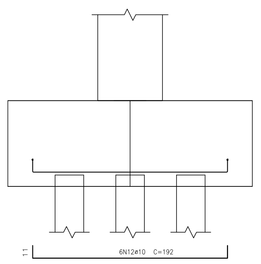
Viga paralela Y





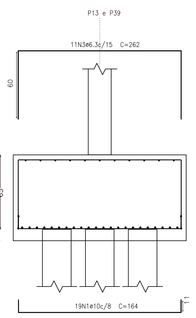
Viga diagonal



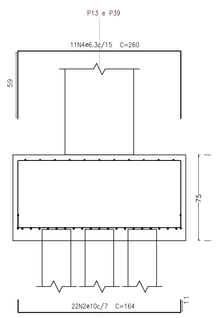


P13 e P39

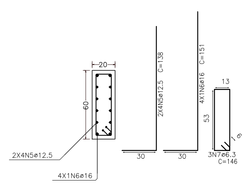
P13 e P39



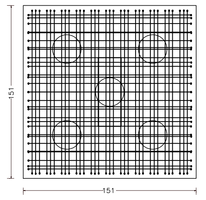
P13 e P39



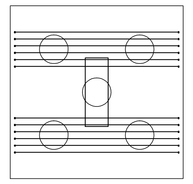
P13 e P39

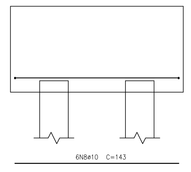


Estacas: D25

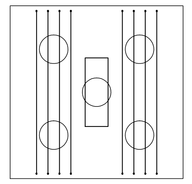


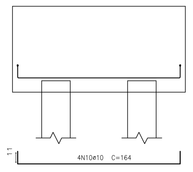
Viga paralela X



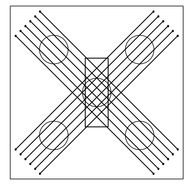


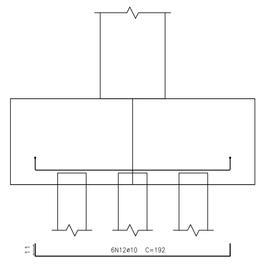
Viga paralela Y





Viga diagonal

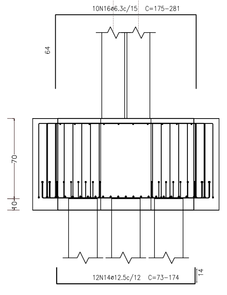




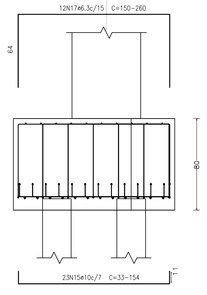
(P11-P12) e (P37-P38)

P12 e P38

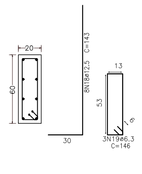
P11 e P37



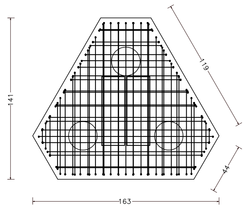
P11 e P37



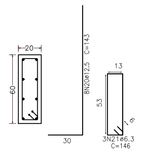
P11 e P37



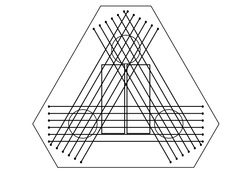
Estacas: D25

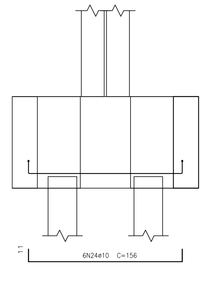


P12 e P38



Viga lateral





Elemento	Pos.	Diam.	Q	Dob (cm)	Reta (cm)	Dob (cm)	Comp (cm)	Total (cm)	CA-50 (kg)
P8=P9=P10=P34 P35=P36	1	e10	19	11	142	11	164	3116	19.2
	2	e10	22	11	142	11	164	3608	22.2
	3	e6.3	11	60	142	60	262	2882	7.1
	4	e6.3	11	59	142	59	260	2860	7.0
	5	e12.5	6	30	108		138	828	8.0
	6	e16	4	30	121		151	604	9.5
	7	e6.3	3		146		146	438	1.1
	8	e10	6		143		143	858	5.3
	9	e10	6		143		143	858	5.3
	10	e10	4	11	142	11	164	656	4.0
	11	e10	4	11	142	11	164	656	4.0
	12	e10	6	11	170	11	192	1152	7.1
	13	e10	6	11	170	11	192	1152	7.1
							Total=	108	117.6
								(x6)	705.6
								e6.3:	100.2
								e10:	489.6
								e12.5:	52.8
								e16:	63.0
								Total:	705.6

Elemento	Pos.	Diam.	Q	Dob (cm)	Reta (cm)	Dob (cm)	Comp (cm)	Total (cm)	CA-50 (kg)
P13=P39	1	e10	19	11	142	11	164	3116	19.2
	2	e10	22	11	142	11	164	3608	22.2
	3	e6.3	11	60	142	60	262	2882	7.1
	4	e6.3	11	59	142	59	260	2860	7.0
	5	e12.5	8	30	108		138	1104	10.6
	6	e16	4	30	121		151	604	9.5
	7	e6.3	3		146		146	438	1.1
	8	e10	6		143		143	858	5.3
	9	e10	6		143		143	858	5.3
	10	e10	4	11	142	11	164	656	4.0
	11	e10	4	11	142	11	164	656	4.0
	12	e10	6	11	170	11	192	1152	7.1
	13	e10	6	11	170	11	192	1152	7.1
							Total=	108	120.5
								(x2)	241.0
(P11-P12)=(P37-P38)	14	e12.5	12		VAR.		VAR.	1596	15.4
	15	e10	23		VAR.		VAR.	2622	16.2
	16	e6.3	10		VAR.		VAR.	2350	5.8
	17	e6.3	12		VAR.		VAR.	2640	6.5
	18	e12.5	8	30	113		143	1144	11.0
	19	e6.3	3		146		146	438	1.1
	20	e12.5	8	30	113		143	1144	11.0
	21	e6.3	3		146		146	438	1.1
	22	e10	6	11	134	11	156	936	5.8
	23	e10	6	11	134	11	156	936	5.8
	24	e10	6	11	134	11	156	936	5.8
							Total=	108	94.1
								(x2):	188.2
ÁREA DE FORMAS = 44,28 M2 VOLUME DE CONCRETO = 18,24 M3								e6.3:	65.4
								e10:	237.2
								e12.5:	105.6
								e16:	21.0
								Total:	429.2

Configuração de Penas
Colorir em TODAS as Penas com: **- Espessura = 0.08** **- Color = Black** **- GrayScale = Off**
Com exceção das seguintes

Pena	Espessura
1	0.10
2	0.20
3	0.30
4	0.40
5	0.50
6	0.60
7	0.15
8	0.05

NOTAS GERAIS

Concreto a usar: fck = 25,0 MPa.
Medidas expressas em centímetros. Escalas indicadas no desenho.
Não tomar medidas do desenho com régua (escala).
Observar comprimentos mínimos de traspasse (emendas e esperas) exigidos.
Obedecer cobrimentos das armaduras segundo as normas da ABNT e o projeto.
Dispor armaduras de pele (costela) quando se indicar no desenho da ferragem.
Jamais alterar a distribuição das armaduras de pilares nas respectivas seções.
Projeto de fundações conforme RELATÓRIO DE SONDAGEM N. 145.SO.2009, fornecido por PROSONDA FUNDAÇÕES LTDA, em 24/11/2009.
No caso de dúvidas, ou na suspeita de engano no desenho, contatar o calculista.

Nº	Comentário	Data	Autor
00	EMISSÃO INICIAL	20/04/20	Manassés

UNIVERSIDADE FEDERAL DO AMAZONAS

Prefeitura da UFAM

CAMPUS: CAMPUS UNIVERSITÁRIO – SETOR NORTE

OBRA: FES – FACULDADE DE ESTUDOS SOCIAIS

TÍTULO: PROJETO DE ESTRUTURA EM CONCRETO ARMADO
BLOCOS DE FUNDAÇÃO

AUTOR DO PROJETO	ART:	DATA	ESCALA	ARQUIVO
Manassés Ibernon Engº Civil – CREA 11116/D-AM	AM20200208122	20/04/20	INDICADA	01-EC-034.DWG

PRANCHA: 01.EC.034